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To:	Mr. Michael Johnson		
	Director of Community and Economic Development		
	City of Cottonwood Heights		
	Mr. Adam Ginsberg		
	Staff Engineer		
	Public Works, City of Cottonwood Heights		
From:	Daniel J. Brown, P.E., Senior Geotechnical Engineer No. 10186640		
	DANIEL J.		
Date:	November 3, 2020		
Subject:	Review of Geotechnical Study and Slope Stability Analysis		
	Proposed Wasatch Rock Development, 6695 South Wasatch Boulevard Cottonwood Heights, Utah (May 13, 2020)		
	Review of Response Letter, Review of Geotechnical Study		
	and Slope Stability Analysis, Proposed Wasatch Rock Development,		
	6695 South Wasatch Boulevard, Cottonwood Heights, Utah (July 9, 2020)		

# Review of Final Slope Stability Analysis, Proposed Wasatch Rock Development 6695 South Wasatch Boulevard, Cottonwood Heights, Utah (October 7, 2020)

#### Introduction

At the request of Mr. Michael Johnson and Mr. Adam Ginsberg, GeoStrata reviewed the subject report for the Wasatch Rock Development completed by Gordon Geotechnical Engineering, Inc. (Gordon Geotechnical) and dated October 7, 2020. The referenced report was submitted to Cottonwood Heights City for a proposed mixed-use development currently planned to include a hotel, a large apartment building, a condominium tower, a senior living center, three mixed-use pads, various ancillary parking areas, three retail pads and re-alignment of Wasatch Boulevard along the western boundary of the subject site. GeoStrata completed a review of a previous version of the subject Gordon Geotechnical report titled "Geotechnical Study and Slope Stability Analysis, Proposed Wasatch Rock Development, 6695 Wasatch Boulevard, Cottonwood Heights, Utah" and dated May 13, 2020 in a review letter dated June 17,2020. A review response was prepared by Gordon Geotechnical in a letter dated July 9, 2020 titled "Response Letter, Review of Geotechnical Study and Slope Stability Analysis, Proposed Wasatch Rock Development, 6695 Wasatch Boulevard, Cottonwood Heights, Utah". Our review of the Gordon Geotechnical October 7, 2020 report was conducted on behalf of Cottonwood Heights City to assist the city in protecting public health, safety, and welfare, and to reduce risks to future property owners. The purposes of our review are to assess whether or not the report adequately addresses the geotechnical and slope stability concerns associated with the project consistent with reasonable standards of practice and in accordance with Cottonwood Heights City's Sensitive Lands Evaluation & Development Standards (SLEDS) (Title 19 Chapter 19.72 of the Cottonwood Heights City Municipal code). The objectives to be achieved by the designation of a sensitive lands district include, without limitation, the following:

A. The protection of the public from natural hazards, such as landslide, rockfall, debris flow, earthquake ground rupture, liquefaction, shallow ground water, snow melt/storm water runoff and erosion.

#### **Review Discussion**

Section 1.3, Objectives and Scope, of the October 7, 2020 Gordon Geotechnical report states:

"The objectives and scope of our study were planned in discussions between Mr. Josh Cowley of Rockworth Companies and Mr. Patrick Emery of  $G^2$ .

In general, the objectives of this study were to:

- 1. Further define and evaluate the general subsurface soil and groundwater conditions across the site.
- 2. Provide updated slope stability recommendations to be utilized in the design and construction of the proposed facilities."

Section 1.5, Professional Statements, of the October 7, 2020 Gordon Geotechnical report states:

"Supporting data upon which our recommendations are based are presented in subsequent sections of this report. Recommendations presented herein are governed by the physical properties of the soils encountered in the exploration borings, test pits, and trenches, measured and projected groundwater conditions, and the layout and design data discussed in Section 2., Proposed Construction, of this report. If subsurface conditions other than those described in this report are encountered and/or if design and layout changes are implemented, G2 must be informed so that our recommendations can be reviewed and amended, if necessary.

Our professional services have been performed, our findings developed, and our recommendations prepared in accordance with generally accepted engineering principles and practices in this area at this time."

The following review comments are from our June 17, 2020 review of the May 13, 2020 Gordon Geotechnical report with responses from the Gordon Geotechnical July 9, 2020 response letter and additional comments as applicable based on our review of the July 9, 2020 response letter and the October 7, 2020 slope stability report.

# June 17, 2020 Review Comment 1

Section 2, Proposed Construction, of the May 13, 2020 Gordon Geotechnical report states:

"Development plans for the site have changed since the previous geotechnical reports for the site. Development at the site is complicated by the existence of several fault lines and a buried aqueduct which render significant portions of the site as "un-buildable" for habitable structures. These fault lines and buried aqueduct have been considered from the onset when designing the latest development plans. The proposed structures have been strategically located in the "buildable" areas defined in the surface fault rupture hazard report. Currently, a hotel, an apartment, condominium, senior living center, three mixed-use pads, and three retail pads are planned for the site. Additionally, Wasatch Boulevard along the western boundary of the site will be re-aligned to bi-sect the site in a generally north-south direction."

GeoStrata understands that a geologic hazards study has been completed for the site concurrently with this geotechnical study, which includes a surface fault rupture hazard assessment. GeoStrata recommends that Cottonwood Heights City request that the GeoStrata review comments regarding the surface fault rupture hazard be addressed prior to approving final buildable areas for the proposed development.

#### **Gordon Geotechnical Response to Review Comment 1**

"A geologic hazard evaluation and surface fault rupture hazard evaluation was performed by Mr. Bill Black of Western Geologic (G2 sub consultant). Comments regarding the surface fault rupture hazard will be addressed in a separate letter by Western Geologic."

#### **Current Comment 1**

An update to portions of the geologic hazards study has been completed with the October 7, 2020 Gordon Geotechnical report. GeoStrata is currently completing a separate review of the updated geologic hazards study. Any deficiencies with the geologic hazards evaluation (if present) will be noted in our current geologic hazards review letter. Any comments pertaining the updated geologic hazards evaluation must be addressed prior to approving final buildable areas for the proposed development

#### June 17, 2020 Review Comment 2

Section 2, Proposed Construction, of the May 13, 2020 Gordon Geotechnical report states:

"Maximum column and wall loads for the condominium structure are anticipated to be very large, on the order of 1,000 to 1,500 kips and 15 to 30 kips per lineal foot, respectively. Detailed structural loads will be needed to finalize geotechnical recommendations for this structure."

Following approval of buildable areas and final subdivision of lots for the subject property, GeoStrata recommends that Cottonwood Heights City request that a design level geotechnical assessment be performed for each of the proposed buildings, which incorporates detailed structural loads. This is especially critical for the proposed condominium structure.

#### **Gordon Geotechnical Response to Review Comment 2**

" $G^2$  is planning to provide a detailed geotechnical assessment for each proposed building with recommendations tailored to the structural loads when foundation plans or loading details become available."

#### **Current Comment 2**

GeoStrata recommends that the City require that a detailed geotechnical assessment be provided for each proposed building with recommendations tailored to the structural loads when foundation plans or loading details become available.

#### June 17, 2020 Review Comment 3

Section 2, Proposed Construction, of the May 13, 2020 Gordon Geotechnical report states:

"Final site grading will require cuts up to 30 feet in the northern portions of the site and fills on the order of 5 to 10 feet in the southwest portion of the site. Fills up to 35 feet in height are planned for the eastern portion of the site and will buttress the existing gravel pit cut slope. Final site grading slopes are generally not anticipated to not exceed 50 percent or 2.0:1.0 (H:V) with localized areas of up to 56 percent or 1.8:1.0 (H:V)."

GeoStrata recommends that Cottonwood Heights City request that the project geotechnical engineer review final site grading plans to assess whether the final grading plan conforms to the slope stability analysis and recommendations presented in the May 13, 2020 Gordon geotechnical report or whether additional slope stability analysis is needed for the subject development.

# Gordon Geotechnical Response to Review Comment 3

" $G^2$  can review final site grading plans for conformance with the slope stability recommendations provided as final site grading plans become available."

# **Current Comment 3**

Section 6, Conclusions, of the October 7, 2020 Gordon Geotechnical report states:

"The slope at Section C-C' is the steepest with an average grade of approximately 53 percent. The stability analysis indicates that compacted bank-run sand and gravel fill material will be required for slopes that exceed 50 percent or 2:1 (H:V)."

Cottonwood Height Code of Ordinances, Title 19, Chapter 19.72, Section 19.72.40, Development Standards and Controls, Subsection F, Cut and Fill Slopes states:

Cut and fill slopes shall comply with the following unless otherwise recommended in an approved soils and geology report:

1. Cut and fill slopes shall not exceed 12 feet.

2. Cut and fill slopes shall not exceed a slope ratio of 2:1 except as follows:

(a) No slopes shall be cut steeper than the bedding plane, fracture, fault or joint in any formation where the cut slope will lie on the dip of the strike line of the fracture, bedding plane, fault or joint.

(b) No slopes shall be cut in an existing landslide, mud flow or other form of naturally unstable slope.

(c) If the material of a slope is of such composition and character as to be unstable under the anticipated maximum moisture conditions, the slope angle shall be reduced to a stable value or increased through retention using a method approved by the city engineer and certified as to its stability by a professional soils engineer.

3. Fill slopes shall not be constructed on natural slopes steeper than 2:1.

4. Roadway cut and fill slopes located outside the dedicated public right-of-way shall be within recorded easements providing for slope protection and preservation. The easements shall be in a form acceptable to the city.

GeoStrata recommends that Cottonwood Heights City request that the project geotechnical engineer

review final site grading plans to assess whether the final grading plan conforms to the final accepted slope stability analysis and recommendations or whether additional slope stability analysis is needed for the subject development.

#### June 17, 2020 Review Comment 4

Section 5.1, Summary of Findings, of the May 13, 2020 Gordon Geotechnical report states:

"The condominium structure at Section A-A' incorporates a deep cut for below-grade parking. A structural element must extend a minimum of 15 feet below the bottom of footings to assure an adequate factor of safety. This may consist of deep foundations, soil improvement, or a permanent shoring solution such as soil nails."

GeoStrata recommends that Cottonwood Heights City request that internal, external, and global stability of the permanent shoring and/or retaining wall to be constructed on the uphill side of the proposed condominium structure be evaluated prior to approval for construction. This evaluation will likely be completed during final design of this structure.

# **Gordon Geotechnical Response to Review Comment 4**

"Internal, external, and global stability analyses will be performed in conjunction with the final design of the condominium structure and required shoring design. Shoring designs are typically provided by the specialty contractors performing the work."

#### **Current Comment 4**

In the October 7, 2020 Gordon Geotechnical report, cross-section A-A' was updated based on additional subsurface data obtained from new boreholes. Section 6, Conclusions, of the October 7, 2020 Gordon Geotechnical report states:

"The condominium structure at Section A-A' incorporates a deep cut for below-grade parking. A structural element must extend a minimum of 5 feet below the bottom of footings on the cut side to assure an adequate factor of safety. This may consist of deep foundations, soil improvement, or a permanent shoring solution such as soil nails. A depth of 5 feet is shallower than previously recommended in the May 13, 2020 report. This is due to the lack of groundwater encountered to depths of 81 feet at boring locations B-9 and B-10. The projected groundwater is deeper in the updated slope stability models resulting in more favorable slope stability conditions."

GeoStrata recommends that Cottonwood Heights City request that internal, external, and global stability of the permanent shoring and/or retaining wall to be constructed on the uphill side of the proposed condominium structure be evaluated prior to approval for construction. This evaluation will likely be completed during final design of this structure and should include and assessment of temporary cut and/or shoring during construction.

In addition, GeoStrata recommends that Cottonwood Heights City request that the constructability of the required cuts for this structure be investigated for feasibility. It is likely that additional geotechnical recommendations will be required for this portion of the project in order to decrease the potential for collapse of larger cuts within the unconsolidated granular sediments.

#### June 17, 2020 Review Comment 5

Section 5.2.2, Geometry, of the May 13, 2020 Gordon Geotechnical report states:

"The geometry for the slope stability models was developed from the geologic cross-sections provided with the concurrent Geologic Hazards Evaluation report. Topography was obtained from 2013 lidar data with 0.5-meter resolution. Three cross-sections (A-A', B-B', and C-C') for slope stability analysis were selected based on the locations of the proposed developments and the most adverse topographic and geologic conditions."

Based on our review of the slope stability results presented in Appendix D and review of the Western Geologic prepared geologic cross sections, the slope stability cross sections closely match the Western Geologic cross sections. If any changes to the geologic cross sections are made based on comments from our review of the Western Geologic geologic hazards study, any updated information, or otherwise, GeoStrata recommends that Cottonwood Heights City request that slope stability cross sections likewise be updated.

#### **Gordon Geotechnical Response to Review Comment 5**

" $G^2$ 's slope stability cross-sections are based on the cross-sections provided by Western Geologic in conjunction with the Geologic Hazard Evaluation report for the project. G2 can update slope stability cross-sections as needed if changes are made in response to the review of the Geologic Hazard Evaluation report."

#### **Current Comment 5**

Section 5.1.2, Geometry, of the October 7, 2020 Gordon Geotechnical report states:

"The geometry for the slope stability models was developed from the geologic cross-sections provided with the concurrent Geologic Hazards Evaluation report. Topography was obtained from 2013 lidar data with 0.5-meter resolution. Four cross-sections (A-A', B-B', C-C', and D-D') for slope stability analysis were selected based on the locations of the proposed developments and the most adverse topographic and geologic conditions."

Based on our review of the slope stability results presented in Appendix D of the October 7, 2020 report and review of the Western Geologic prepared geologic cross sections presented in Appendix A of the October 7, 2020 report, the updated slope stability cross sections closely match the updated Western Geologic cross sections. If any changes to the geologic cross sections are made based on comments from our review of the Western Geologic geologic hazards study, any updated information, or otherwise, GeoStrata recommends that Cottonwood Heights City request that slope stability cross sections likewise be updated.

#### June 17, 2020 Review Comment 6

Section 5.2.3, Soil Strength, of the May 13, 2020 Gordon Geotechnical report states:

"The soil parameters were selected for analysis based upon direct shear test results performed on undisturbed and laboratory recompacted samples. Strength parameters for the more coarsegrained granular soils were selected based upon our experience with similar soils in the area. These coarse-grained sand and gravel soils are projected to exhibit relatively high strengths based on their performance history in gravel pit cut slopes which have been known to stand near vertical for extended periods of time. The cohesive characteristic of these granular soils may be explained by a slight cementation and interlocking of particles. Parameters of concrete washout are estimated as a hybrid between high strength soil and low-grade concrete."

Slope cross section B-B' includes a large area of 'Inferred Extents of Concrete Washout'. As the extents of this material are unknown, GeoStrata recommends that Cottonwood Heights City request that reduced strength parameters be applied to this soil unit. In our opinion, the reduced strength parameters for the 'Concrete Washout' material should be similar to the 'Site Grading Fill' soil strength parameters in order to account for the unknown extents and soil of the materials.

#### Gordon Geotechnical Response to Review Comment 6

" $G^2$  has performed an updated slope stability analysis for Cross-Section B-B' considering reduced strength parameters to account for the unknown extents of the concrete washout material. The results of the stability analysis indicate that the slope stability recommendations provided are still valid. Graphical results of the updated slope stability analysis are included with Appendix A of this letter."

#### **Current Comment 6**

In both the July 9, 2020 Gordon Geotechnical response letter and in the October 7, 2020 Gordon Geotechnical report, strength parameters of the 'Concrete Washout' were updated to match the strength parameters of the 'Site Grading Fill' as requested in our June 17, 2020 review letter. This portion of the comment may be considered addressed.

However, in Section 6.0 "Conclusions" of the October 7, 2020 G<sup>2</sup> report, it states "Section B-B' indicates that concrete washout material may remain in place provided that any loose or raveling material is removed and the concrete washout is competent."

GeoStrata recommends that Cottonwood Heights City request clarification concerning how the competency of this material is to be assessed, and when such assessments would need to be completed or that the concrete washout material be considered undocumented fill which will require removal, replacement, and compaction to rebuild the slope to satisfactory conditions.

#### June 17, 2020 Review Comment 7

Section 5.2.4, Analysis Results, of the May 13, 2020 Gordon Geotechnical report lists recommendations for site grading.

GeoStrata recommends that Cottonwood Heights City request that the applicant include these recommendations and detail drawings for benching of fill slopes into the existing slope with the final grading plan.

#### **Gordon Geotechnical Response to Review Comment 7**

" $G^2$  can assist in providing a detail drawing for benching of the fill slopes into the existing slopes as final site grading plans are developed."

#### **Current Comment 7**

Section 6, Conclusions, of the October 7, 2020 Gordon Geotechnical report lists recommendations for site grading.

GeoStrata recommends that Cottonwood Heights City request that the applicant include these recommendations and detail drawings for benching of fill slopes into the existing slope with the final grading plan.

#### June 17, 2020 Review Comment 8

Cottonwood Height Code of Ordinances, Title 19, Chapter 19.72, Appendix C, Section 3.0, Submittals, Subsection (c) states:

"The results of any slope stability analyses must be submitted with pertinent backup documentation (i.e., calculations, computer output, etc.). Printouts of input data, output data (if requested), and graphical plots must be submitted for each computer-aided slope stability analysis."

GeoStrata recommends that Cottonwood Heights City request that printouts of input/output data for the near-surface and global slope stability analysis calculations be provided as required by the Cottonwood Heights Code of Ordinances.

# **Gordon Geotechnical Response to Review Comment 8**

"Data printouts and calculations for the near-surface and global stability analysis calculations are provided with Appendix B of this letter."

#### **Current Comment 8**

Printouts of input/output data for near-surface and global slope stability analysis have been included in the July 9, 2020 Gordon Geotechnical response letter and in the October 7, 2020 Gordon Geotechnical report. This comment may be considered addressed.

#### June 17, 2020 Review Comment 9

Section 5.3.1, Site Preparation, of the May 13, 2020 Gordon Geotechnical report states:

"Preparation of the site must consist of the removal of all non-engineered fills, loose surficial soils, topsoil, debris, and other deleterious materials from beneath an area extending at least five feet beyond the perimeter of the proposed building, rigid pavement, and exterior flatwork areas.

The non-engineered fills may remain in flexible pavement areas as long as they are properly prepared. Proper preparation will consist of scarifying and moisture conditioning the upper eight inches and recompacting to the requirements of structural fill. However, it should be noted that compaction of fine-grained soils (if encountered) as structural site grading fill will be very difficult, if not impossible, during wet and cold periods of the year. As an option for proper preparation and recompaction, the upper eight inches of the non-engineered fills may be removed and replaced with granular subbase over proofrolled subgrade. Even with proper preparation, flexible pavements established on non-engineered fills may experience some

longterm movements. If the possibility of these movements is not acceptable, these nonengineered fills must be completely removed."

GeoStrata recommends that Cottonwood Heights City request that the recommendation to not fully remove non-engineered fills from beneath flexible pavements be modified to only include private parking and driveway areas. All city roads will require all undocumented or non-engineered fills be fully removed beneath pavements.

#### **Gordon Geotechnical Response to Review Comment 9**

G2 recommends that the Cottonwood Heights City requirements for City roads be followed. City roads must be established upon structural site grading fill extending to suitable natural soils. The recommendations provided for flexible pavement site preparation in our May 13, 2020 report are still valid for all private parking and roadway areas.

# **Current Comment 9**

This comment may be considered addressed.

# June 17, 2020 Review Comment 10

Section 5.4.2, Reinforced Continuous Mat, of the May 13, 2020 Gordon Geotechnical report states:

"We request that a bearing pressure distribution plan be provided to our office for review, when available."

As part of final design of the proposed structures and as recommended in Review Comment 2, GeoStrata recommends that Cottonwood Heights City request that a design level geotechnical assessment be performed for each of the proposed buildings, which incorporates detailed structural loads. As part of preparation of these reports, a review of planned building foundations and foundation loads should be completed by the geotechnical engineer.

#### **Gordon Geotechnical Response to Review Comment 10**

" $G^2$  can provide a detailed geotechnical assessment for each proposed building, including the condominium structure, when final foundation loading details become available."

#### **Current Comment 10**

As part of final design of the proposed structures and as recommended in Review Comment 2, GeoStrata recommends that Cottonwood Heights City request that a design level geotechnical assessment be performed for each of the proposed buildings, which incorporates detailed structural loads. As part of preparation of these reports, a review of planned building foundations and foundation loads should be completed by the geotechnical engineer.

#### June 17, 2020 Review Comment 11

Cottonwood Height Code of Ordinances, Title 19, Chapter 19.72, Appendix D, Section 7.0, Submittals, Subsection (c) states:

"The results of any liquefaction analyses must be submitted with pertinent backup documentation (i.e., calculations, computer output, etc.). Printouts of input data, output data (on request), and

graphical plots must be submitted for each computer aided liquefaction analysis. In addition, input data files, recorded on diskettes, CDs, or other electronic media, may be requested to facilitate the city's review."

GeoStrata recommends that Cottonwood Heights City request that printouts of input/output data for liquefaction hazard analysis calculations be provided as required by the Cottonwood Heights Code of Ordinances.

#### **Gordon Geotechnical Response to Review Comment 11**

"Data printouts for the liquefaction analysis are provided with Appendix C of this letter."

#### **Current Comment 11**

Printouts of input/output data for liquefactions hazard analysis were provided in the July 9, 2020 Gordon Geotechnical response letter. This comment may be considered addressed.

#### **Current Comment 12**

Section 5.1.3, Soil Strength, of the October 7, 2020 Gordon Geotechnical report states:

"The soil parameters were selected for analysis based upon direct shear test results performed on undisturbed and laboratory recompacted samples. Strength parameters for the more coarsegrained granular soils were selected based upon our experience with similar soils in the area. These coarse-grained sand and gravel soils are projected to exhibit relatively high strengths based on their performance history in gravel pit cut slopes which have been known to stand nearvertical for extended periods of time. The cohesive characteristic and friction angle of these granular soils may be explained by a slight cementation and interlocking of particles. Strength parameters of concrete washout are estimated as being equal to that of site grading fill.

The table on the following page summarizes the soil strength values utilized for static and seismic conditions."

Soil Type	Soil Parameter	Parameter Units
	Cohesion	200 (psf)
Lacustrine Sand and Gravel	Friction Angle	36
	Unit Weight	120 (pcf)
Laminated Silty Fine	Cohesion	0 (psf)
Sand Beds, Poorly Graded Sand Beds,	Friction Angle	33
Lacustrine Sand	Unit Weight	120 (pcf)
	Cohesion	150 (psf)
Lacustrine Fines	Friction Angle	27
	Unit Weight	120 (pcf)
Site Grading Fill	Cohesion	350 (psf)
(Compacted Washout Fines)	Friction Angle	33
	Unit Weight	120 (pcf)
Site Grading Fill	Cohesion	250 (psf)
(Compacted Sand and Gravel)	Friction Angle	38
	Unit Weight	120 (pcf)
	Cohesion	350 (psf)
Concrete Washout	Friction Angle	33
	Unit Weight	130 (pcf)
	Cohesion	0 (psf)
Pre-Lake Bonneville Landslide Deposits	Friction Angle	21
	Unit Weight	120 (pcf)
	Cohesion	200 (psf)
Pre-Lake Bonneville Alluvial Fan Deposits	Friction Angle	36
	Unit Weight	120 (pcf)

The table above lists soil strength parameters utilized in the slope stability assessment for the proposed development. GeoStrata recommends that Cottonwood Heights City request that the consultant add a column to this table to list the source of each of these soil strength parameters, i.e. sample location, depth, etc.

While responding to this comment, it is recommended that the Consultant note Section 3.0 (a) *Submittals* of Title 19.72 of the Cottonwood Heights City Code, which states; "*Submittals for review shall include boring logs; geologic cross sections; trench and test pit logs; laboratory data (particularly shear strength test results, including individual stress-deformation plots from direct shear tests); discussions pertaining to how idealized subsurface conditions and shear strength parameters used for analyses were developed; analytical results, and summaries of the slope stability and conclusions regarding slope stability. Section 6.0 (a) of the Cottonwood City Code states; "Adequate evaluation of slope stability for a given site requires thorough and comprehensive geologic and geotechnical engineering studies. These* 

studies are a crucial component in the evaluation of slope stability. Geologic mapping and subsurface exploration are normal parts of field investigation. Samples of earth materials are routinely obtained during subsurface exploration for geotechnical testing in the laboratory to determine the shear strength and other pertinent engineering properties.

It should be noted that all soil strength parameters need to be based either on laboratory testing completed on representative samples of the soils being investigated, or on correlations between other laboratory or field investigation parameters (such as SPT blow counts). Soil strength parameters based on "experience with similar soils in the area" are not considered to represent appropriate sources of soil strength parameters.

# Closure

This review letter is issued in response to the consultant's assessment of the above referenced site. Comments and recommendations in this review letter are based on field data presented by the Consultant. GeoStrata has not performed an independent site assessment. GeoStrata has relied on the Consultant's reports in performing its services. Consequently, it does not represent or warrant that the Consultant's report contains accurate data or proper recommendations. Recommendations and Comments presented in this review letter are provided to Cottonwood Heights City to aid in reducing risks from geologic hazards. GeoStrata makes no warranty; either expressed or implied and shall not be liable for any direct, special, incidental, or consequential damages with respect to claims by users of this review.

All services performed by GeoStrata for this review were provided for the exclusive use and benefit of Cottonwood Heights City. No other person or entity is entitled to rely on GeoStrata's services or use the information contained in this letter without the express written consent of GeoStrata.

If there are any questions concerning the contents of this review, please feel free to contact our office at (801) 501-0583.